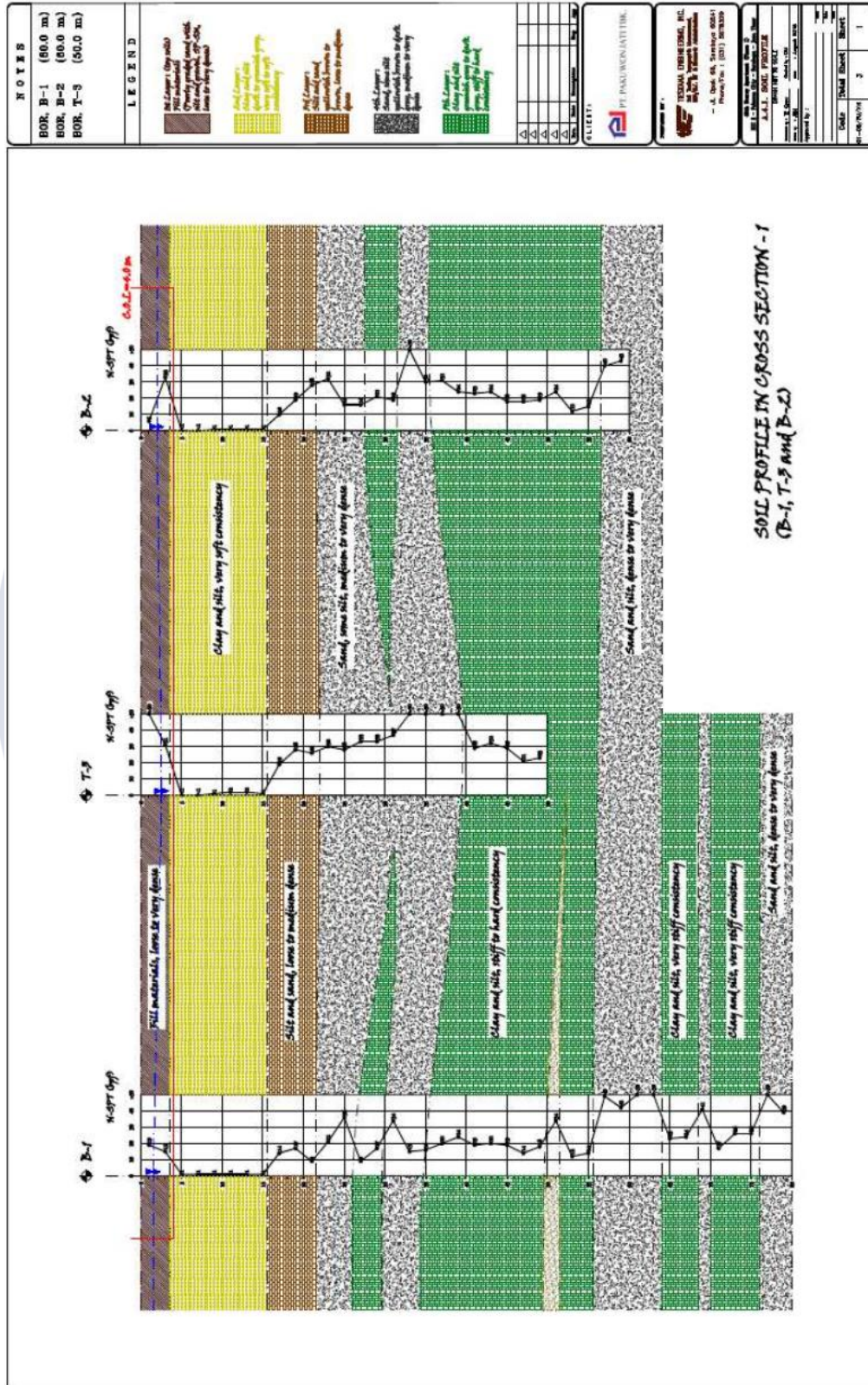


## DAFTAR PUSTAKA

- Alawiah, W. A., & Yakin, Y. A. (2016). Analisis Daya Dukung Tiang Tunggal Statik Pada Tanah Lunak di Gedebage. Institut Teknologi Nasional, Vol.2 No.2. Bandung.
- Bjerrum, L., dan Simons, N. E. (1960). Comparison of Shear Strength Characteristic of Normally Consolidated Clay. Research Conference on Shear Strength of Cohesive Soils.
- Bowles, J. E. (1989). Sifat-sifat Fisis dan Geoteknis Tanah,
- Bowles, J. E. (1991). Analisa dan Desain Pondasi, Edisi keempat Jilid 1. Erlangga.
- Bowles, Joseph E. (1997). Foundation Analysis and Design Fifth Edition. The McGraw-Hill Companies, Inc. Singapore.
- CV. Testana Engineering, Surabaya. (2016). "Soil Investigation. Apartement ECC Mansion. Pakuwon City".
- Das, Braja M. (1995). Mekanika Tanah 1. Erlangga. Jakarta
- Das, Braja M. (1941). Soil Mechanics Laboratory Manual Third Edition. United States of America: Engineering Press, Inc.
- Hardiyatmo, H. C. (2002). Mekanika Tanah I, Gadjah Mada University Press, Yogyakarta.
- Hekmatyar, I., Fauzy, I., DA, I., & Sadono, K. W. (2017). Analisa Perilaku Daya Dukung Tiang Tunggal dengan Rumus Statik dan Model Fisik pada Tanah Pasir.
- Klar, (1999) *Faculty of Civil Engineering, Technion – Israel Institute of Technology, Israel (Geotechnical Engineering 159 Issue GE3).*
- Lim, A. (2014). Evaluasi Formula Penentuan Daya Dukung Aksial Tiang Pancang Tunggal Menggunakan Data CPT Berdasarkan Metode Langsung (Direct Method). Bandung: Universitas Katolik Parahyangan.
- Mayerhof G. G. (1965). Shallow Foundation. Journal ASCE, Soil Mechanic Foundation Diy, vol.91. No. SM2. 66.
- Mhaiskar, SY, G Khare, Makarand, Vaidya, Ravikiran. (2010). "High Strain Dynamic Pile Testing and Static Load Test – A correlation Study", Indian Geotechnical Conference, IGS Mumbai Chapter & IIT Bombay, India.

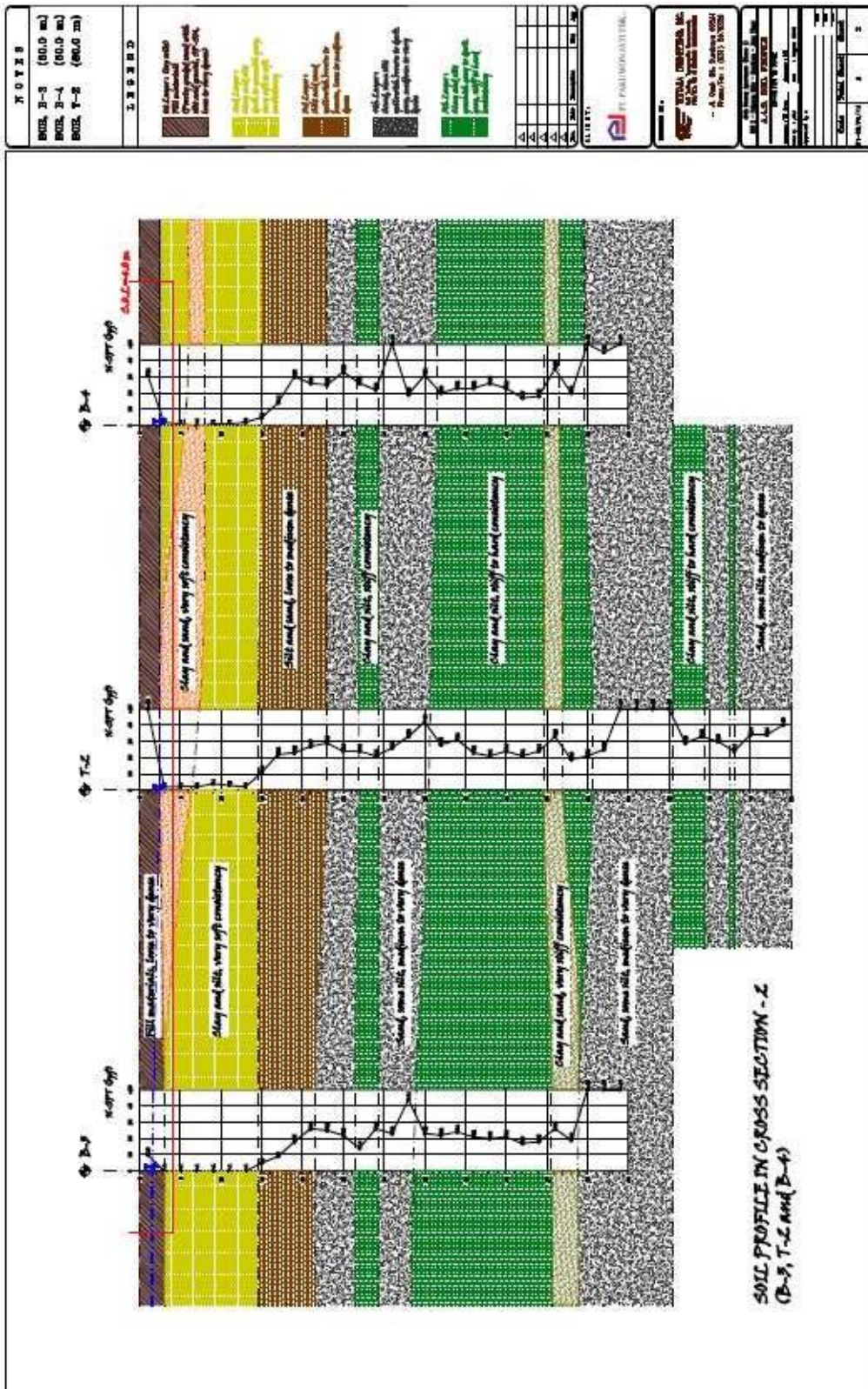
- PT. Geotechnical Engineering Consultant.Bandung. (2019). “Instrumentasi Fiber Optik pada uji Pembebanan Static (TP-02)”.Jakarta.
- PT. Erlangga. Jakarta. Coduto, D. P. (1994). “Foundation Design, Principles and Practices”, Prentice Hall International, Inc., New Jersey. Suhaimi, M., Fathurrozi., & Rahman, M. A. (2017). Perbandingan Daya Dukung Ultimit Tiang Pancang antara Metode Teoritis dan Metode Aktual dengan Konfigurasi Tiang dan Kedalaman. Gradasi Teknik Sipil, Vol 1 No.2.
- Surjandari, N. S. (2008). Studi Perbandingan Perhitungan Daya Dukung Aksial fondasi Tiang Bor Menggunakan Uji Beban Statik dan Metode Dinamik. Media Teknik Sipil. Surakarta.
- Sosrodarsono, Dan Kazuto Nakazawa, (1981). Mekanika Tanah & Teknik Pondasi, PT Pradnya Paramita, Jakarta.
- Terzaghi, K., and Peck, R. B. (1948). Soil Mechanics in Engineering Practice. Wiley, New York.
- Terzaghi, K. and Peck, R. B. (1967). Soil Mechanics in Engineering Practice. John Wiley & Sons, Inc., New York.
- Reese, L.C. & Wright, S.J.(1977), “Drilled Shaft Design and Construction Guidelines Manual” vol.1, U.S. Department of Transportation.
- Raharjo, P.P. & Salim,(1995) “Aplikasi Program Komputer BORPILE untuk Perhitungan Daya Dukung Aksial Pondasi Tiang Bor.” Seminar Rekayasa Pondasi, “Beneficial Use of Bored Pile to Support Heavy Loadings,” Surabaya.

Lampiran 1. Irisan tanah 1 (data penyelidikan tanah B-1, T-3 dan B-2),



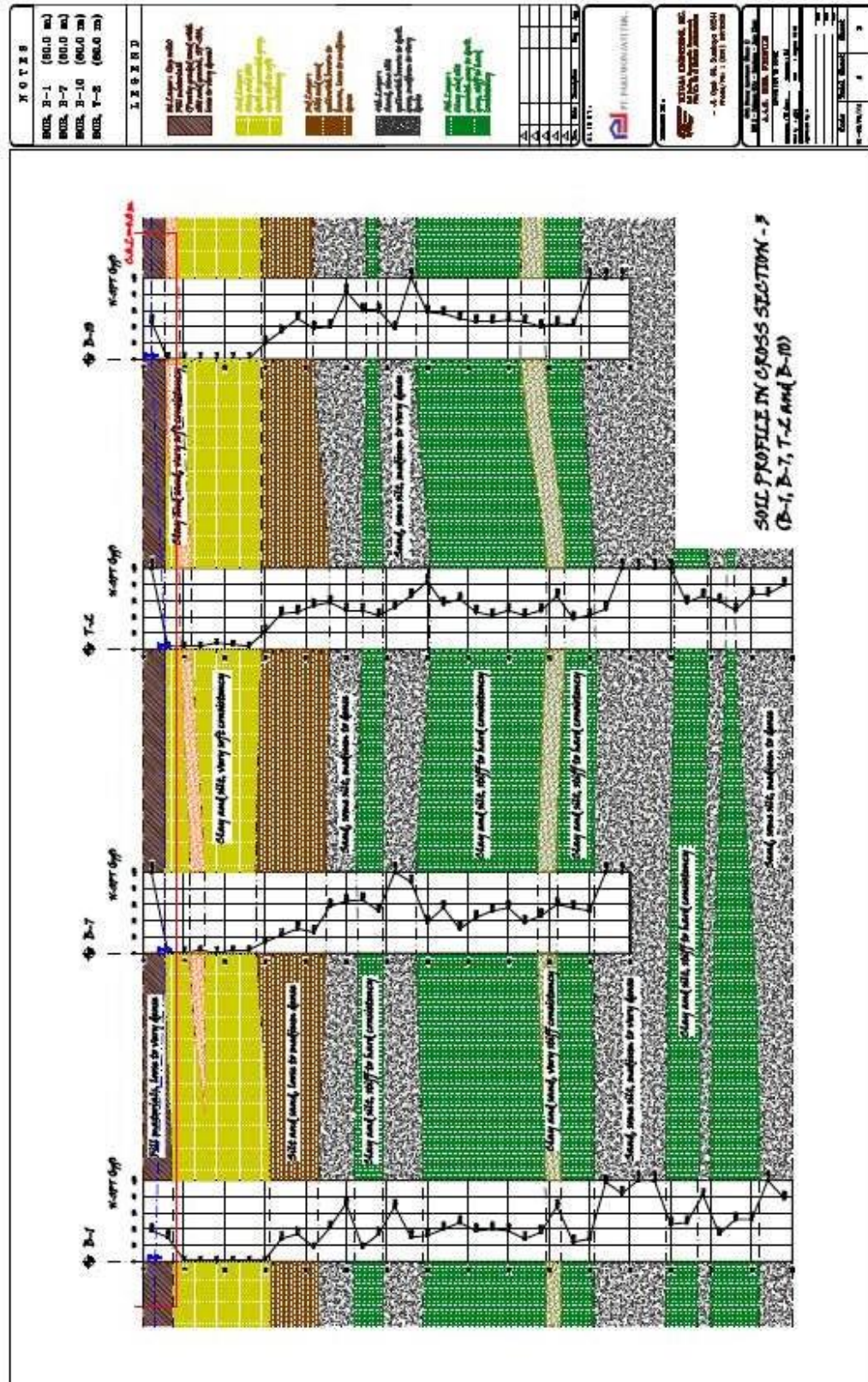


Lampiran 2. Irisan tanah 2 (data penyelidikan tanah B-3, T-2, dan B-4),





Lampiran 3. Irisan tanah 3 (data penyelidikan tanah B-1, B-7, T-2 dan B-10)



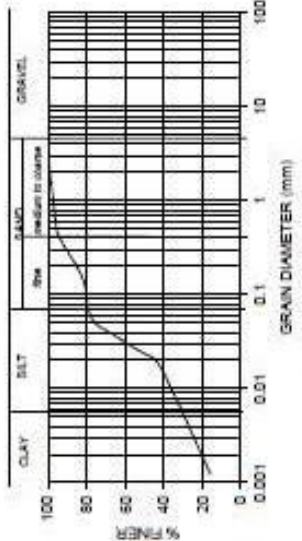
# Lampiran 4. Grainsize Distribution,

## GRAIN SIZE DISTRIBUTION

Project : Apartment.  
 Location : ECC Mansion, Pakuwon City - Surabaya.  
 Sieve Analysis

**Hydrometer**

Sieve opening No. (mm)	mean weight	% retained	% Cumulative retained	% finer	Hydrometer			Effective depth L (cm)	Hyd. Cor. only for mercuric (R)	% finer	L <sub>A</sub>	K	D, mm
					Time (min)	Actual Hyd. reading (Ra)	Hyd. reading (Rc)						
1.6"	38.100	0.00	0.00	100.00	0.25	51	48.8	7.8	52	79.02	31.200	0.01248	0.070
1.0"	25.400	0.00	0.00	100.00	0.5	49	46.8	8.1	50	75.78	18.200	0.01248	0.050
3/4"	19.000	0.00	0.00	100.00	1	43	40.8	9.1	44	68.06	8.100	0.01248	0.038
3/8"	9.500	0.00	0.00	100.00	2	37	34.8	10.1	38	56.35	5.050	0.01248	0.028
4	4.750	0.00	0.00	100.00	3	33	31.8	10.7	34	49.87	3.967	0.01248	0.024
8	2.360	1.65	0.55	98.45	4	30	27.8	11.2	31	45.01	2.800	0.01248	0.021
16	1.180	5.24	1.75	97.70	5	27	24.8	11.7	28	40.16	1.683	0.01248	0.015
30	0.600	4.92	1.54	98.06	16	25	22.8	12	26	38.02	0.750	0.01248	0.011
40	0.425	4.48	1.46	94.57	30	23	20.8	12.4	24	33.68	0.413	0.01248	0.008
100	0.150	37.54	12.51	82.06	60	21	18.8	12.7	22	30.44	0.212	0.01248	0.006
200	0.075	6.84	2.21	20.16	120	19	16.8	13	20	27.20	0.108	0.01248	0.004
TOTAL	60.47	20.38	-	-	24 hrs	12	9.8	14.2	13	15.87	0.010	0.01248	0.001



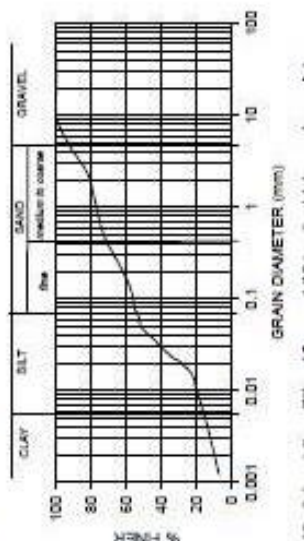
Visual description : Clay and silt (CL), dark grey, trace of fine sand

## GRAIN SIZE DISTRIBUTION

Project : Apartment.  
 Location : ECC Mansion, Pakuwon City - Surabaya.  
 Sieve Analysis

**Hydrometer**

Sieve opening No. (mm)	mean weight	% retained	% Cumulative retained	% finer	Hydrometer			Effective depth L (cm)	Hyd. Cor. only for mercuric (R)	% finer	L <sub>A</sub>	K	D, mm
					Time (min)	Actual Hyd. reading (Ra)	Hyd. reading (Rc)						
1.6"	38.100	0.00	0.00	100.00	0.25	51	48.8	7.8	52	53.52	31.200	0.01212	0.068
1.0"	25.400	0.00	0.00	100.00	0.5	49	46.8	8.1	50	51.53	18.200	0.01212	0.049
3/4"	19.000	0.00	0.00	100.00	1	43	40.8	9.1	44	44.75	8.100	0.01212	0.037
3/8"	9.500	0.00	0.00	100.00	2	37	34.8	10.1	38	38.17	5.050	0.01212	0.027
4	4.750	21.94	7.51	92.89	3	33	30.8	10.7	34	33.78	3.967	0.01212	0.023
8	2.360	30.87	10.26	82.40	4	29	26.8	11.4	30	29.38	2.800	0.01212	0.020
16	1.180	13.14	4.38	78.02	8	25	20.8	12.4	24	22.81	1.590	0.01212	0.015
30	0.600	11.04	5.68	25.96	16	21	18.8	12.7	22	20.62	0.704	0.01212	0.011
40	0.425	6.88	2.95	28.62	30	19	16.8	13	20	18.43	0.433	0.01212	0.008
100	0.150	37.17	12.39	41.01	60	17	14.8	13.3	18	19.23	0.202	0.01212	0.006
200	0.075	11.80	3.69	44.94	120	15	12.8	13.7	16	14.04	0.114	0.01212	0.004
TOTAL	134.92	44.94	-	-	24 hrs	9	8.8	14.7	10	7.48	0.010	0.01212	0.001



Visual description : Silt and fine sand (ML), yellowish brown, trace of clay



## Lampiran 5. Strength Test,

### VANE SHEAR TEST

Project : Apartment.  
Location : ECC Mansion, Pakuwon City, Surabaya.

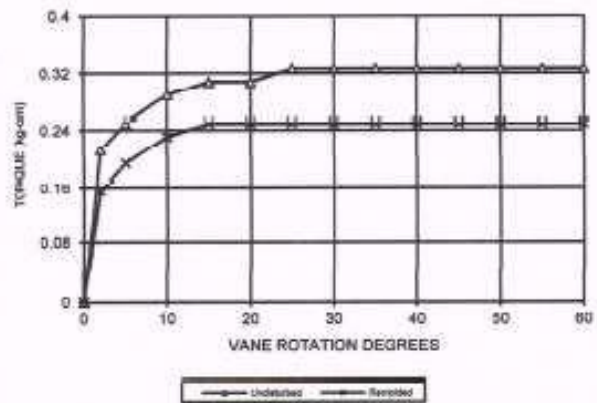
Bore Hole no : T-2  
Depth : 6.00-6.50 m  
Apparatus of vane  
Vane height (H) : 1.25 cm  
Vane width (D) : 1.25 cm  
Spring no : 2

Tested by : BR.  
Checked by : DM.

Soil description : Clay and silt

Sample Max  $S_u$ , kg/cm<sup>2</sup>  
UD : 0.08  
R : 0.06  
Sensitivity : 1.31

Vane Rotation Degrees ( $^{\circ}$ )	Spring deflection degrees		Torque T kg.cm	
	UD	R	UD	R
0	0.0	0.0	0.000	0.000
2	11.0	8.0	0.212	0.164
5	13.0	10.0	0.209	0.182
10	15.0	12.0	0.288	0.231
15	16.0	13.0	0.308	0.250
25	18.5	15.5	0.308	0.250
30	17.0	13.5	0.327	0.250
35	17.0	13.5	0.327	0.250
40	17.0	13.5	0.327	0.250
45	17.0	13.5	0.327	0.250
50	17.0	13.5	0.327	0.250
55	17.0	13.5	0.327	0.250
60	17.0	13.5	0.327	0.250



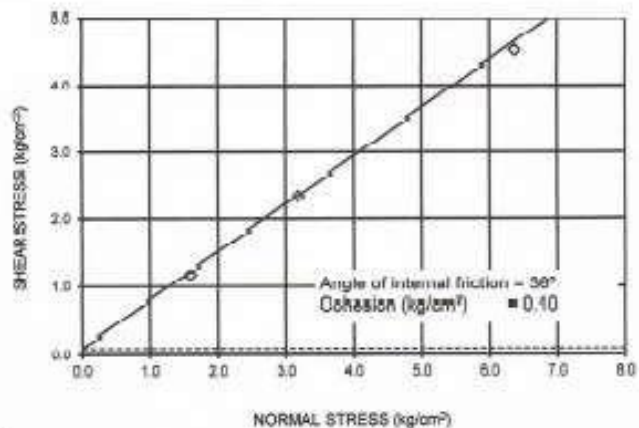
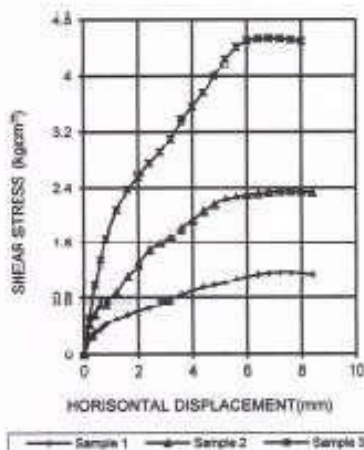
### DIRECT SHEAR TEST

Project : Apartment.  
Location : ECC Mansion, Pakuwon City,  
Surabaya.

Boring no. : T-2  
Depth : 18.00-18.50 m  
Date of test : Juni 2018

Operator : BR.  
Checked by : DM

Sample no.	1	2	3
Type	Quick	Quick	Quick
Normal stress, kg/cm <sup>2</sup>	1.55	3.18	6.37
Shear stress, kg/cm <sup>2</sup>	1.189	2.356	4.501

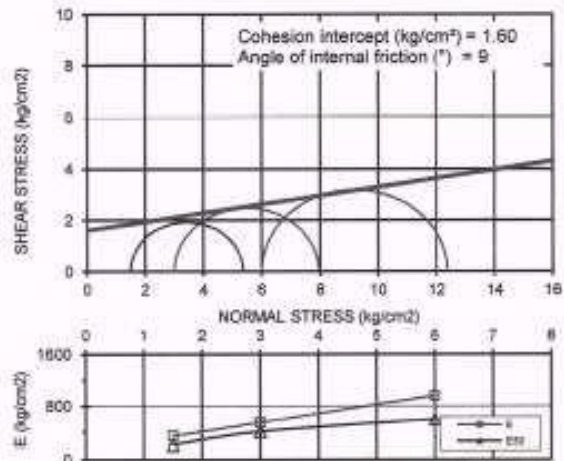
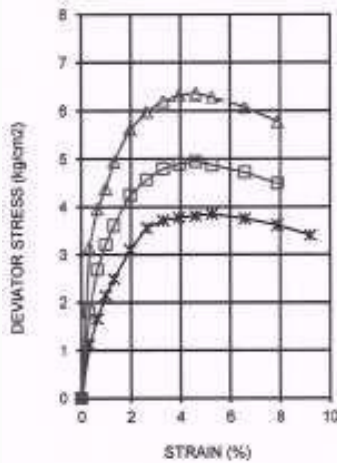


### TRIAXIAL COMPRESSION TEST

Project : Apartment  
 Location : ECC Mansion, Pakuwon City,  
 Surabaya.

Date of Testing : Juni 2016  
 Boring no. : T-2  
 Sample depth : 36.00-36.50 m

Sample no.	1	2	3
Type	UU	UU	UU
Lateral stress (kg/cm <sup>2</sup> )	1.80	3.00	8.00
Deviator stress (kg/cm <sup>2</sup> )	3.85	4.95	6.37
Strain at failure, %	5.26	4.61	4.61
Modulus of elasticity (kg/cm <sup>2</sup> )	341.98	547.17	957.55
Modulus secant (kg/cm <sup>2</sup> )	217.42	409.02	598.90

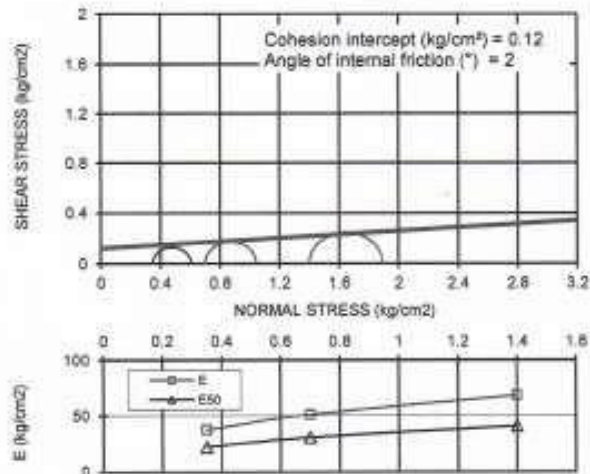
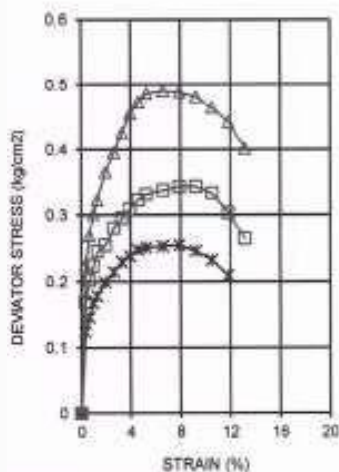


### TRIAXIAL COMPRESSION TEST

Project : Apartment  
 Location : ECC Mansion, Pakuwon City,  
 Surabaya.

Date of Testing : Juni 2016  
 Boring no. : T-3  
 Sample depth : 8.00-8.50 m

Sample no.	1	2	3
Type	UU	UU	UU
Lateral stress (kg/cm <sup>2</sup> )	0.35	0.70	1.40
Deviator stress (kg/cm <sup>2</sup> )	0.25	0.34	0.49
Strain at failure, %	7.89	9.21	6.58
Modulus of elasticity (kg/cm <sup>2</sup> )	37.62	51.30	88.40
Modulus secant (kg/cm <sup>2</sup> )	22.16	30.68	40.90





## Lampiran 6. Consolidation Test

### CONSOLIDATION TEST

Project : Apartment.  
 Location : ECC Mansion, Pakuwon City, Surabaya.  
 Boring No : T-2  
 Depth : 6.00-6.50 m

Test No : -  
 Date : Juni 2018  
 Tested by : DR  
 Checked by : DM

Apparatus  
 ring height, cm = 1.92  
 diameter, cm = 5.95  
 Area, cm<sup>2</sup> = 27.81

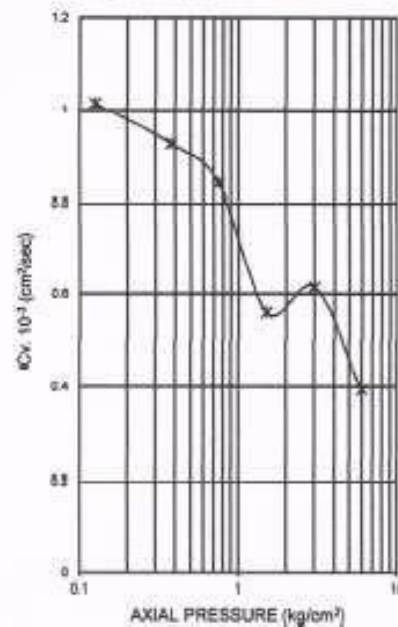
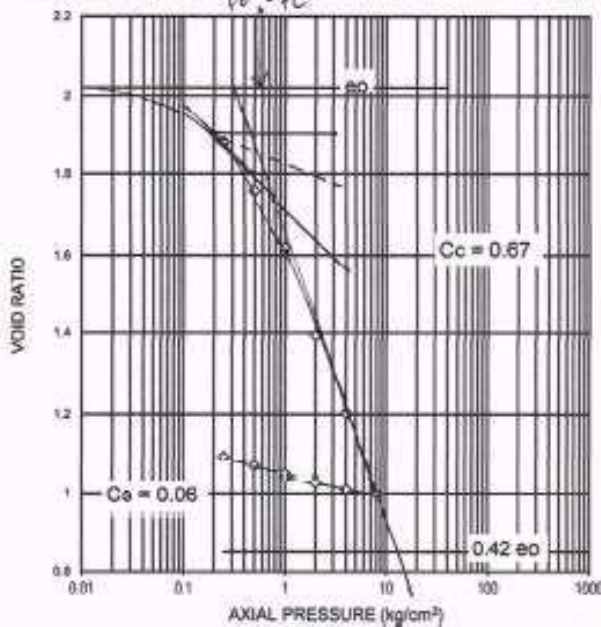
Gs = 2.58  
 eo = 2.02

Ws, solid weight (gr) = 45.59  
 ZHo, solid height (cm) = 0.64

Applied pressure P (kg/cm <sup>2</sup> )	Final dial reading	Dial change	Height 2H (cm)	Void height 2H-2Ho	$e = \frac{2H-2Ho}{2Ho}$	Fitting time (sec)	CV, Coef of consolidation
0.00	1.000	0.000	1.920	1.285	2.021		
0.25	0.910	0.078	1.830	1.195	1.880	735.00	1.01E-03
0.40	0.899	0.078	1.792	1.117	1.787	735.00	9.25E-04
1.00	0.745	0.087	1.685	1.030	1.620	735.00	8.42E-04
2.00	0.600	0.140	1.520	0.880	1.392	960.00	5.60E-04
4.00	0.480	0.120	1.400	0.785	1.203	735.00	6.15E-04
6.00	0.380	0.130	1.270	0.605	0.808	960.00	3.94E-04
4.00	0.358	-0.008	1.278	0.643	1.011		
2.00	0.370	-0.010	1.290	0.655	1.030		
1.00	0.380	-0.010	1.300	0.665	1.048		
0.40	0.390	-0.013	1.310	0.670	1.068		
0.25	0.410	-0.017	1.330	0.695	1.093		

Liquid Limit (%) : 42  
 Plasticity Index (%) : 20  
 Specific Gravity : 2.58  
 Preconsolidation pressure (kg/cm<sup>2</sup>) : 1.56  
 Effective overburden pressure (kg/cm<sup>2</sup>) : 0.66

Sample height (cm) : 1.92  
 Water content (%) : 78.0  
 Dry unit weight (tn/m<sup>3</sup>) : 0.85  
 Void ratio : 2.02  
 Saturation : 100



## Lampiran 7.. Isotropic Compression Parameter.

### ISOTROPIC COMPRESSION PARAMETER

Project : Apartment.  
 Location : ECC Mansion, Pakuwon City, Surabaya.  
 Boring No : T-2  
 Depth : 6.00-6.50 m

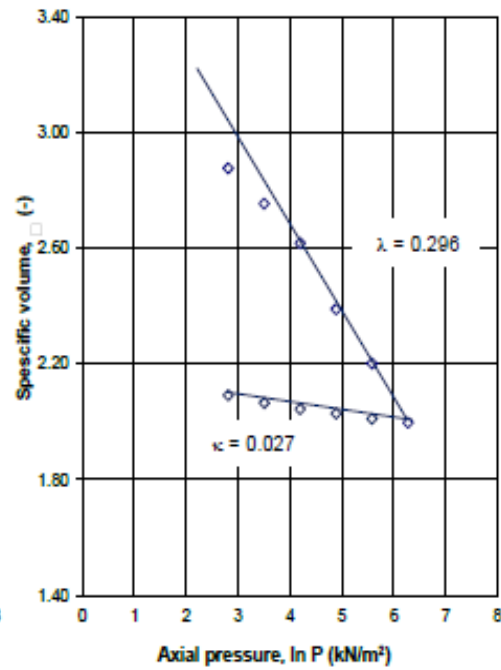
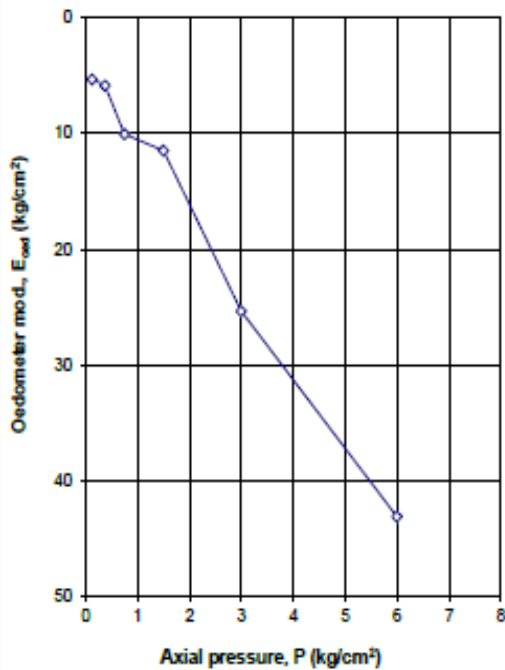
Test No : -  
 Date : Juni 2016  
 Tested by : BR  
 Checked by : DM

Aparatus  
 ring height, cm = 1.92  
 diameter, cm = 5.95  
 A area, cm<sup>2</sup> = 27.81

Gs = 2.58  
 eo = 2.02

Ws, solid weight (gr) = 45.59  
 2H<sub>o</sub>, solid height (cm) = 0.64  
 Effective overburden pressure (kg/cm<sup>2</sup>) = 0.56

Applied pressure P (kg/cm <sup>2</sup> )	Final dial reading	Dial change	Height 2H (cm)	Void height 2H-2H <sub>o</sub>	e = $\frac{2H-2H_0}{2H_0}$	$\Delta e$	$\frac{\Delta h}{h}$	K (cm/s)	Coed (kg/cm <sup>2</sup> )	In P (kN/m <sup>2</sup> )	v (-)
0.00	1.000	0.090	1.920	1.285	2.021	0.142	0.047	1.90E-07	5.33		
0.25	0.910	0.078	1.830	1.195	1.880	0.123	0.043	1.58E-07	5.87	2.813	2.880
0.50	0.832	0.087	1.752	1.117	1.757	0.137	0.050	8.36E-08	10.07	3.507	2.757
1.00	0.745	0.145	1.665	1.030	1.620	0.228	0.087	4.88E-08	11.48	4.200	2.620
2.00	0.600	0.120	1.520	0.885	1.392	0.189	0.079	2.43E-08	25.33	4.893	2.392
4.00	0.480	0.130	1.400	0.765	1.203	0.205	0.093	9.14E-09	43.08	5.586	2.203
8.00	0.350	-0.008	1.270	0.635	0.998					6.279	1.998
4.00	0.358	-0.012	1.278	0.643	1.011					5.586	2.011
2.00	0.370	-0.010	1.290	0.655	1.030					4.893	2.030
1.00	0.380	-0.013	1.300	0.665	1.046					4.200	2.046
0.50	0.393	-0.017	1.313	0.678	1.066					3.507	2.066
0.25	0.410		1.330	0.695	1.093					2.813	2.093





## Lampiran 8. Hasil pengujian di laboratorium

Summary of Laboratory Test Results

Borehole ID	m	Soil Type	USCS Classification	Index Properties						Engineering Properties																	
				$\gamma$ t/m <sup>3</sup>	G <sub>s</sub>	Void Ratio	Sr %	L <sub>L</sub> %	W <sub>n</sub> %	P <sub>L</sub> %	I <sub>L</sub>	Type of Strength Test	Cohesion kg/cm <sup>2</sup> (total stress)	$\phi$ °	E <sub>u</sub> kg/cm <sup>2</sup> kg/cm <sup>2</sup>	Un drained Moduli	E <sub>50</sub> kg/cm <sup>2</sup> kg/cm <sup>2</sup>	Oedometer Moduli	C <sub>c</sub>	C <sub>s</sub>	Swelling Index	Preconsolidation Pressure kg/cm <sup>2</sup>	OCR	Modified compression index	Modified swelling index	$\kappa$	
T-2	6.0 - 6.5	Clay and silt, dark grey	CL	1.52	2.58	2.02	100	42	78	22	2.80	Vane	0.08	-	8	0.67	0.06	0.36	1.00	0.296	0.027						
	18.0 - 18.5	Silt and fine sand, yellowish brown	ML	1.83	2.67	1.00	100	45	37	34	0.27	QT	0.10	36	39	0.28	0.03	2.00	1.28	0.095	0.011						
	36.0 - 36.5	Clay and silt, dark grey	CH	1.77	2.58	1.06	100	77	41	30	0.23	UU	1.60	9	547	409	0.32	1.50	0.51	0.138	0.021						
T-3	8.0 - 8.5	Clay and silt, dark grey	CH	1.50	2.52	2.04	100	54	81	23	1.87	UU	0.12	2	50	30	0.75	0.90	1.38	0.318	0.041						
	20.0 - 20.5	Silt and sand, brown	ML	1.87	2.60	0.84	100	43	32	28	0.27	QT	0.01	37	30	0.43	0.09	2.80	1.66	0.158	0.040						
	40.0 - 40.5	Clay and silt, yellowish brown	CH	1.83	2.61	0.93	100	60	35	29	0.19	UU	1.70	4	600	400	0.35	1.10	1.19	0.150	0.049						
B-1	4.0 - 4.5	Clay and silt, dark grey	CH	1.52	2.54	1.97	100	74	78	33	1.10	UU	0.12	8	45	30	12	1.10	3.06	0.349	0.057						
	14.0 - 14.5	Clay and silt, dark grey	CH	1.51	2.50	1.95	100	66	78	26	1.30	UU	0.12	4	60	60	13	1.30	1.49	0.322	0.050						
	66.0 - 66.5	Clay and silt, greenish grey	CH	1.76	2.60	1.10	100	105	42	38	0.06	UU	1.40	3	270	170	0.42	6.00	1.24	0.167	0.062						
B-2	16.0 - 16.5	Clay and silt, greenish grey	CH	1.55	2.56	1.81	100	114	70	41	0.40	UU	0.23	4	100	70	30	0.63	0.19	2.00	1.85	0.221	0.084				
	46.0 - 46.5	Clay and silt, yellowish brown	CL	1.67	2.65	1.44	100	48	54	26	1.27	UU	1.15	4	410	225	24	0.56	0.10	0.85	0.28	0.250	0.045				
	52.0 - 52.5	Clay and silt, greenish grey	CH	1.65	2.62	1.46	100	72	55	33	0.56	UU	1.10	5	440	280	40	0.54	0.14	4.00	1.15	0.222	0.063				
B-3	8.0 - 8.5	Clay and silt, dark grey	CH	1.52	2.53	1.91	100	60	75	25	1.43	UU	0.11	3	40	27	6	0.72	0.11	0.40	0.70	0.331	0.052				
	24.0 - 24.5	Sand, some silt yellowish brown	SM	1.88	2.68	0.85	94	35	30	27	0.38	QT	0.25	35	70	0.22	0.03	2.50	1.27	0.085	0.011						
	44.0 - 44.5	Clay and silt, yellowish brown	CH	1.71	2.58	1.20	100	52	46	27	0.76	UU	1.50	9	270	170	32	0.41	0.11	1.50	0.44	0.170	0.050				
B-4	10.0 - 10.5	Clay and silt, dark grey	CH	1.51	2.60	2.13	100	61	82	28	1.64	Vane	0.08	-	8	0.77	0.13	0.60	0.79	0.310	0.055						
	28.0 - 28.5	Clay and silt, yellowish brown	CH	1.70	2.59	1.27	100	93	49	36	0.23	UU	1.35	12	530	330	21	0.60	0.15	2.10	1.04	0.265	0.067				
	54.0 - 54.5	Clay and silt, dark grey	CH	1.66	2.59	1.40	100	64	54	27	0.73	UU	1.20	9	554	363	45	0.40	0.07	2.90	0.78	0.173	0.035				
B-7	14.0 - 14.5	Clay and silt, greenish grey	CH	1.55	2.53	1.74	100	60	68	28	1.25	UU	0.42	3	140	81	9	0.62	0.11	0.80	0.78	0.311	0.052				
	26.0 - 26.5	Sand and silt, brown	SM	1.91	2.70	0.84	97	37	30	26	0.36	QT	0.40	38	30	0.34	0.05	2.50	1.18	0.169	0.024						
	42.0 - 42.5	Silt and clay, yellowish brown	ML	1.79	2.61	1.04	100	46	40	28	0.67	UU	1.90	5	450	200	50	0.35	0.07	3.90	1.15	0.152	0.033				
B-8	6.0 - 6.5	Clay and silt, dark grey	CL	1.48	2.52	2.12	100	48	83	27	2.67	Vane	0.08	-	7	0.69	0.08	0.60	1.20	0.247	0.036						
	12.0 - 12.5	Clay and silt, dark grey	CH	1.51	2.50	1.95	100	66	78	32	1.35	UU	0.10	4	60	60	6	0.76	0.15	0.40	0.50	0.368	0.068				
	46.0 - 46.5	Silt and fine sand, yellowish brown	ML	1.86	2.70	0.97	100	44	36	30	0.43	QT	0.25	35	70	0.33	0.03	3.73	1.00	0.148	0.014						
B-9	12.0 - 12.5	Clay and silt, dark grey	CH	1.47	2.50	2.15	100	75	85	34	1.24	Vane	0.08	-	11	0.66	0.09	0.91	1.00	0.276	0.041						
	28.0 - 28.5	Clay and silt, yellowish brown	CH	1.66	2.58	1.39	100	79	54	33	0.46	UU	1.18	5	240	130	130	0.26	0.08	3.80	1.93	0.113	0.035				
	40.0 - 40.5	Clay and silt, yellowish brown	CH	1.75	2.61	1.13	100	57	43	27	0.53	UU	1.40	6	360	205	53	0.40	0.10	3.00	1.05	0.164	0.048				
B-10	4.0 - 4.5	Clay and sand, dark grey	CL	1.75	2.55	1.07	100	44	42	21	0.91	QT	0.16	31	11	0.30	0.04	0.70	1.75	0.121	0.016						
	22.0 - 22.5	Sand and silt, brown	SM	1.85	2.75	0.98	93	40	33	27	0.46	QT	0.25	35	45	0.26	0.03	1.50	0.78	0.108	0.015						
	48.0 - 48.5	Clay and fine sand, yellowish brown	CH	1.79	2.57	0.98	100	51	38	26	0.48	UU	1.10	8	500	300	60	0.40	0.09	4.40	1.11	0.176	0.041				

## Lampiran 9, Kalibrasi Kabel Fiber Optik

Product inspection report

品名 Product Name	盒式基架应变传感光纤		
型号 Model	NZS-DSS-009	直径 Diameter	Φ5.0mm
数量 (m) Number	1522	纤芯数量 Fiber Quantity	1
布喇格中心频率 (GHz) Brillouin Center Frequency	10.795	纤芯类型 Cable Type	单模(G652B) Single Mode
平均损耗(dB/km) The Average Loss	0.26	弯曲半径 (mm) Curvature Radius	18
应变系数 Strain Coefficient	0.04998 MHz/10 <sup>-6</sup>	温度系数 Temperature Coefficient	1.775 MHz/°C
检验员 Inspector	NZ-02	检验结果 Test Results	合格 Qualified
生产日期 Production Date	2016.03.18	检验日期 Inspection Date	2016.03.22
备注 Note			

附图：初始应变分布图 (Fig the initial strain distribution)

G.B.S (0740.9)

Sumber: Hasil Analisis.